



# A Study On The Structural Behaviour Of RCC-Steel Hybrid Systems Under Lateral Loads

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## ARTICLE INFO

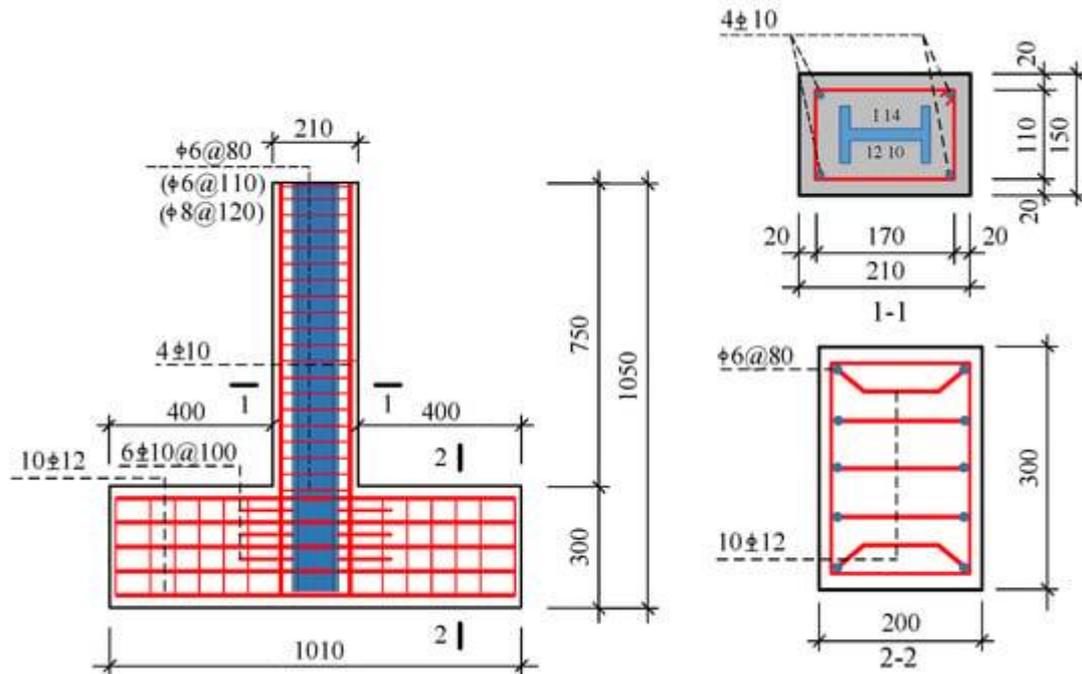
## ABSTRACT

*The behaviour of RCC-Steel hybrid systems under lateral loads plays a vital role in ensuring the safety and resilience of modern infrastructure, particularly in earthquake-prone areas. This study focuses on analysing the lateral load behaviour of RCC-Steel hybrid systems, specifically evaluating storey shear, stiffness, drift, and displacement. By comparing two structural models, T-40-ii and T-40-ii-PLATE, the research investigates the effects of steel plate integration on load distribution, rigidity, and deformation patterns. Emphasizing the lateral response rather than structural health monitoring (SHM) or machine learning (ML) techniques, the study provides valuable insights into how steel plates enhance energy dissipation and improve seismic resistance. The results reveal that incorporating steel plates increases structural stability and minimizes deformation risks, contributing to the design of more durable and resilient hybrid structures suitable for earthquake-resistant infrastructure.*

**Keywords:** RCC-Steel Hybrid Structures, Lateral Load Analysis, Structural Health Monitoring (SHM), Machine Learning in Structural Engineering, Seismic Performance Evaluation

## 1. Introduction:

The structural behaviour of RCC-Steel hybrid systems under lateral loads has become an area of significant research interest due to the unique combination of ductility, strength, and stiffness provided by integrating reinforced cement concrete (RCC) with structural steel components. Understanding the complex interactions between these materials under lateral forces, such as seismic or wind loads, is critical for enhancing structural resilience and safety. Recent advancements in structural analysis and machine learning have enhanced the evaluation of building lateral load systems. Abdeljaber et al. (2017) demonstrated the effectiveness of 1-D Convolutional Neural Networks (CNNs) in capturing non-linear behaviour caused by lateral forces. Similarly, Avci et al. (2021) highlighted how machine learning (ML) and deep learning (DL) models outperform traditional methods in predicting dynamic responses. These approaches are particularly valuable for RCC-Steel hybrid systems, where material heterogeneity and complex load paths challenge conventional analysis, focusing solely on lateral load behaviour without involving structural health monitoring (SHM).



**Figure 1. Cross-sectional dimensions and distributed steels of specimens (unit: mm)**

Source: <https://www.mdpi.com/2076-3417/9/4/687>

Moreover, the role of hysteretic damping in the energy dissipation of hybrid structures has been a focal point in recent studies. Bajrić and Høgsberg (2018) emphasized the significance of accurate damping estimation using stochastic subspace identification techniques, which are essential for predicting the non-linear response of hybrid systems under cyclic lateral loading. In addition, data-driven models, such as Support Vector Machines (SVMs) optimized for damage detection, as presented by Gui et al. (2017), further enhance the predictive capabilities in SHM applications. The integration of advanced data science techniques, as reviewed by Bao et al. (2019), not only improves the accuracy of structural assessments but also facilitates real-time monitoring, enabling proactive maintenance strategies. As lateral loads pose significant risks to the integrity of RCC-Steel hybrid systems, leveraging these modern analytical and computational tools is vital for developing robust design frameworks and improving the overall safety and longevity of such structures.

**1.1 Objectives:**

1. To Evaluate Seismic Performance.
2. To Compare Structural Models.
3. To Enhance Design Strategies for Resilience.

**2. Review of literature:**

Sr. No	Name	Year	Aim	Objectives	Findings of the Study	DOI
1	Abdeljaber, O., Avci, O., Kiranyaz, M. S., Boashash, B., Sodano, H., & Inman, D. J.	2017	To develop 1-D CNNs for effective structural damage detection using benchmark data.	Validate CNN performance in identifying structural damage from SHM data.	1-D CNNs accurately detected structural damage, confirming their effectiveness in SHM.	Link
2	Avci, O., Abdeljaber, O., Kiranyaz, S., Hussein, M., Gabbouj, M., & Inman, D. J.	2021	To review traditional and modern vibration-based damage detection methods in civil structures.	Compare traditional, Machine Learning, and Deep Learning techniques in damage detection.	Machine Learning and Deep Learning significantly enhance vibration-based damage detection methods.	Link
3	Bajrić, A., & Høgsberg, J.	2018	To estimate hysteretic damping in structures using stochastic subspace identification.	Develop a reliable method for estimating damping characteristics in complex structures.	Stochastic subspace identification provides accurate damping estimates	Link

					in structural analysis.	
4	Bao, Y., Chen, Z., Wei, S., Xu, Y., Tang, Z., & Li, H.	2019	To explore the role of data science and engineering in structural health monitoring.	Identify trends and challenges in data-driven structural health monitoring systems.	Data-driven approaches significantly improve monitoring accuracy and decision-making in SHM.	Link
5	Gui, G., Pan, H., Lin, Z., Li, Y., & Yuan, Z.	2017	To implement SVM with optimization techniques for efficient structural health monitoring and damage detection.	Optimize SVM algorithms to improve damage detection accuracy and efficiency.	Optimized SVM models effectively identified structural damages with improved prediction rates.	Link
6	Hakim, S. J. S., Abdul Razak, H., & Ravanfar, S. A.	2016	To diagnose faults in beam-like structures using artificial neural networks.	Enhance fault detection in beam-like structures through AI.	Artificial neural networks effectively identified faults in beam structures.	Link
7	He, Y., Yang, J. P., & Li, Y.-F.	2022	To develop a three-stage framework for automated modal identification of bridge parameters.	Improve accuracy in modal identification despite frequency uncertainties.	The three-stage framework enhanced modal identification accuracy.	Link
8	Hernández-González, I. A., García-Macías, E., Costante, G., & Ubertini, F.	2024	To implement AI-driven blind source separation for fast operational modal analysis.	Accelerate operational modal analysis using AI techniques.	AI methods significantly reduced computation time in modal analysis.	Link
9	Hou, R., & Xia, Y.	2021	To review advancements in vibration-based damage identification in civil structures from 2010-2019.	Summarize recent techniques and challenges in vibration-based damage detection.	Vibration-based methods showed increased efficiency with ML integration.	Link
10	Kiranyaz, S., Avci, O., Abdeljaber, O., Ince, T., Gabbouj, M., & Inman, D. J.	2021	To survey applications of 1D convolutional neural networks in various fields.	Review 1D CNN applications across engineering fields.	1D CNNs demonstrated versatility in multiple engineering applications.	Link
11	Kita, A., Cavalagli, N., & Ubertini, F.	2019	To analyse temperature effects on the static and dynamic behaviour of Consoli Palace.	Assess structural behaviour changes due to temperature variations.	Temperature changes impact both static and dynamic responses.	Link
12	Li, J., Bao, T., & Ventura, C. E.	2021	To create an automated operational modal analysis algorithm for concrete dams.	Simplify operational modal analysis for large-scale concrete structures.	The automated algorithm improved dam safety assessments.	Link
13	Liu, Y., Cao, R., Xu, S., & Deng, L.	2023	To develop a deep learning-based method for structural modal analysis using computer vision.	Utilize computer vision for automated modal analysis.	Deep learning models achieved high accuracy in modal analysis.	Link
14	Lucà, F., Manzoni, S., Cigada, A., & Frate, L.	2021	To propose a vibration-based health monitoring approach for tie-rods	Develop reliable health monitoring under environmental variability.	The proposed method reliably detected tie-rod health issues.	Link

			under uncertain conditions.			
15	Malekloo, A., Ozer, E., & Girolami, M.	2021	To provide an overview of machine learning applications in structural health monitoring.	Highlight the role of emerging technologies in SHM.	ML techniques enhanced SHM effectiveness with high-dimensional data.	Link
16	Park, H. S., & Oh, B. K.	2024	To utilize CNNs for model updating using dynamic structural response data.	Improve model accuracy through CNN-based updating techniques.	CNNs provided accurate structural updates in dynamic conditions.	Link
17	Santos, A., Figueiredo, E., Silva, M. F. M., Sales, C. S., & Costa, J. C. W. A.	2016	To apply kernel-based machine learning algorithms for damage detection.	Optimize kernel methods for efficient structural damage detection.	Kernel-based methods improved damage detection precision.	Link
18	Yun, D. Y., Oh, B. K., Park, K., & Park, H. S.	2024	To identify modal damping ratios in buildings using an LSTM-based approach.	Apply LSTM networks for stable damping ratio identification.	LSTM networks stabilized damping ratio predictions.	Link
19	Zhang, Q., Cai, X., Zhong, Y., Tang, X., & Wang, T.	2024	To predict dynamic responses of high-speed trains on cable-stayed bridges using AI.	Integrate AI methods for predictive modeling of dynamic responses.	AI integration enhanced predictive capabilities for bridge dynamics.	Link
20	Zhang, Y., Wang, L., & Xiang, Z.	2012	To detect structural damage using mode shape squares extracted from a passing vehicle.	Validate mode shape-based damage detection techniques.	Mode shape analysis successfully identified structural damages.	Link

### 2.1 Research gap:

Despite significant advancements in structural health monitoring (SHM) and damage detection using artificial intelligence (AI) and machine learning (ML) techniques, notable research gaps remain. Current studies predominantly focus on specific structural types or controlled environments, often neglecting the complex interactions present in real-world conditions, such as varying environmental factors and multi-hazard impacts. Additionally, while deep learning models like CNNs and LSTMs have enhanced accuracy in damage detection, challenges persist in their generalization and adaptability to different structural configurations and loading scenarios. Moreover, the integration of AI-driven methods with real-time data processing for large-scale structures remains underexplored, limiting their practical implementation in continuous monitoring systems. Bridging these gaps is essential for developing more robust, adaptable, and scalable SHM solutions that can operate effectively under diverse and dynamic conditions.

### 3. Research methodology:

The research methodology for this data analysis focuses on evaluating the seismic performance of RCC-Steel hybrid structures by systematically comparing two structural models, T-40-ii and T-40-ii-PLATE. The study involves the collection of key structural data, including storey shear, storey stiffness, storey drift, and storey displacement across all storeys, from the terrace to the basement levels. These parameters were analysed under lateral loading conditions in both X and Y directions to assess the structures' ability to withstand seismic forces. A comparative approach was employed to examine the impact of incorporating plates into the design, with graphical representations created to visualize differences in load distribution, rigidity, and deformation patterns. The analysis aimed to evaluate the models' lateral load resistance, stability, and compliance with seismic safety standards, with particular attention given to storey drift and displacement limits. The findings were then interpreted to identify performance trends, structural vulnerabilities, and improvements introduced by the addition of plates, offering insights into optimizing hybrid structural designs for enhanced seismic resilience and overall safety.

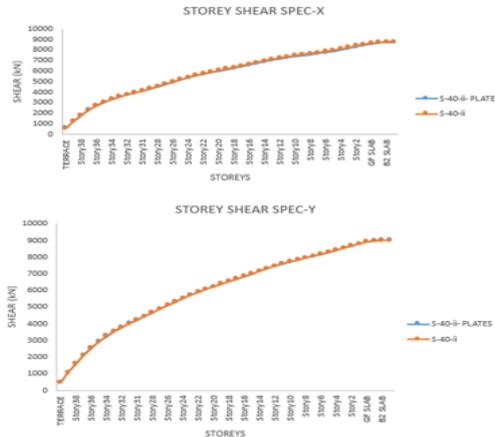
### 4. Data analysis and interpretation:

The data analysis focuses on evaluating the seismic performance of two structural models, T-40-ii and T-40-ii-PLATE, by examining key parameters such as storey shear, stiffness, drift, and displacement. By comparing

these models under lateral loading conditions, the analysis aims to assess the impact of plate incorporation on the overall structural behaviour. This comparative study helps identify improvements in load distribution, structural stability, and seismic resilience, providing insights into optimizing design strategies for enhanced performance during seismic events.

Storey Shear Analysis for S-40-ii and S-40-ii-PLATE Structures

	S-40-ii		S-40-ii-PLATE	
	SPEC X	SPEC Y	SPEC X	SPEC Y
TERrace	549.6757	482.5307	566.3198	494.0942
Storey29	1196.31	958.450	1206.707	976.1193
Storey28	1767.072	1308.09	1806.637	1637.867
Storey27	2261.055	1656.209	2303	2131.01
Storey26	2676.772	1959.933	2717.173	2555.722
Storey25	3000.009	2209.696	3055.436	2844.793
Storey24	3230	2393.869	3305.949	3071.491
Storey23	3537.073	2626.96	3593.051	3353.749
Storey22	3741.831	2779.293	3848.924	3600.126
Storey21	3931.072	4011.581	3954.888	4026.339
Storey20	4121.073	4231.859	4195.729	4243.675
Storey19	4311.074	4449.129	4417.639	4456.483
Storey18	4502.075	4665.731	4594.347	4671.934
Storey17	4708.076	4882.333	4816.257	4890.965
Storey16	4976.076	5109.44	4942.925	5110.9
Storey15	5194.076	5321.909	5162.85	5327.401
Storey14	5400.076	5532.839	5380.043	5535.463
Storey13	5598.076	5726.244	5544.126	5731.223
Storey12	5798.076	5907.859	5709.868	5912.847
Storey11	5913.886	6077.851	5898.01	6080.954
Storey10	6058.916	6237.006	5996.369	6238.956
Storey9	6200.144	6380.196	6131.446	6389.723
Storey8	6343.393	6514.018	6263.01	6529.119
Storey7	6481	6633.141	6414.833	6663.839
Storey6	6646.933	6848.021	6585.901	6845.089
Storey5	6806.946	7055.836	6722.939	7002.767
Storey4	6951.953	7256.565	6878.744	7160.74
Storey3	7100.673	7386.34	7027.377	7319.185
Storey2	7247.278	7482.986	7162.984	7461.913
Storey1	7388.634	7559.826	7286.957	7598.229
GF SLAB	7478.127	7725.974	7398.771	7723.19
Storey29	7570.438	7542.735	7477.205	7539.363
Storey28	7660.934	7953.410	7563.646	7946.931
Storey27	7754.436	8082.436	7653.966	8064.478
Storey26	7850.223	8174.827	7744.802	8165.887
Storey25	7948.436	8259.141	7836.253	8264.144
Storey24	8048.055	8419.686	8007.169	8412.267
Storey23	8248.223	8550.916	8154.902	8545.719
Storey22	8448.436	8676.436	8303.636	8674.267
Storey21	8648.634	8804.446	8450.311	8804.749
GF SLAB	8848.836	8932.456	8598.986	8935.231
Storey29	8748.126	8974.376	8668.339	8990.453
Storey28	8754.436	9002.213	8740.486	9024.41
TE.FLT.LV	9256.364	9003.524	9173.056	9006.436



Interpretation:

The storey shear analysis shows a consistent increase in shear forces from the terrace to the base, with the S-40-ii-PLATE model exhibiting slightly higher values in both SPEC-X and SPEC-Y directions. The inclusion of plates enhances load distribution and overall structural stability, particularly in the lower storeys.

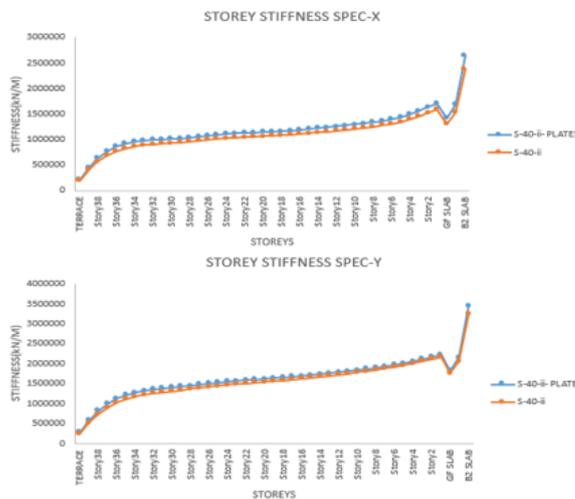
Findings:

The S-40-ii-PLATE model demonstrates improved seismic performance with greater shear resistance, highlighting its enhanced capacity to withstand lateral loads. This suggests that the addition of plates strengthens structural resilience, making it more effective for earthquake-prone regions.

The analysis compares the storey shear performance of S-40-ii and S-40-ii-PLATE square structures under lateral loads. Both models show increasing shear from terrace to base, with the S-40-ii-PLATE showing slightly higher values, indicating better load distribution and enhanced stability, especially in lower storeys.

Storey Stiffness Analysis for S-40-ii and S-40-ii-PLATE Structures

	S-40-ii		S-40-ii-PLATE	
	SPEC X	SPEC Y	SPEC X	SPEC Y
TERrace	89717.11	243238.8	236034.3	272607.3
Storey29	40257.3	81584	44300.32	87576.3
Storey28	56407.2	73109.1	62336.1	81119.3
Storey27	68538.2	89637.3	76364.3	98908.3
Storey26	11178.3	91908	67173.3	111330
Storey25	573837.6	100895.3	57932.3	120000.3
Storey24	867024.6	1174310	956863	1273536
Storey23	639466	1279460	110505.4	1379000
Storey22	504424.6	1257754	899332.6	1382079
Storey21	99537.5	1386594	999972.2	1378037
Storey20	33669	1317636	300444	1413761
Storey19	338837.4	1336357	3021672	1423710
Storey18	95423.0	1367247	3071195	1447411
Storey17	97265.8	1366291	306481	1472529
Storey16	930604.2	1414535	3073329	1437579
Storey15	907489	1446206	3090794	1452240
Storey14	926411	1464472	310904	1469512
Storey13	904873	1489874	3118309	1486697
Storey12	949780	1507742	3127611	1504478
Storey11	965950	1525806	3136913	1522359
Storey10	981675	1543520	3146241	1540223
Storey9	997263	1561895	3155589	1558087
Storey8	1012717	1579950	3164961	1575951
Storey7	1028249	1597611	3174349	1593815
Storey6	1043761	1615896	3183761	1611679
Storey5	1059253	1633832	3193197	1629543
Storey4	1074725	1651429	3202657	1647407
Storey3	1090177	1668696	3212141	1665271
Storey2	1105609	1685652	3221649	1683135
Storey1	1121021	1702309	3231181	1701000
GF SLAB	1136413	1718666	3240737	1718864
Storey29	1141783	1734723	3250317	1736728
Storey28	1157135	1750480	3260000	1754592
Storey27	1172469	1765937	3269697	1772456
Storey26	1187783	1781094	3279409	1790320
Storey25	1203077	1795951	3289136	1808184
Storey24	1218351	1810508	3298879	1826048
Storey23	1233605	1824765	3308637	1843912
Storey22	1248839	1838722	3318410	1861776
Storey21	1264053	1852379	3328197	1879640
Storey20	1279247	1865736	3338000	1897504
Storey19	1294421	1878793	3347819	1915368
Storey18	1309575	1891550	3357653	1933232
Storey17	1324709	1904007	3367502	1951096
Storey16	1339823	1916164	3377366	1968960
Storey15	1354917	1927921	3387245	1986824
Storey14	1370001	1939278	3397139	2004688
Storey13	1385075	1950235	3407048	2022552
Storey12	1399939	1960792	3416972	2040416
Storey11	1414793	1970949	3426911	2058280
Storey10	1429627	1980706	3436865	2076144
Storey9	1444441	1990063	3446834	2094008
Storey8	1459235	1999020	3456818	2111872
Storey7	1474009	2007577	3466817	2129736
Storey6	1488763	2015734	3476831	2147600
Storey5	1503497	2023491	3486860	2165464
Storey4	1518211	2030848	3496904	2183328
Storey3	1532905	2037805	3506963	2201192
Storey2	1547579	2044362	3517037	2219056
Storey1	1562233	2050519	3527126	2236920
GF SLAB	1576867	2056276	3537230	2254784
TE.FLT.LV	2065552	2064107	2036714	3447771



Interpretation:

The storey stiffness analysis reveals that the S-40-ii-PLATE structure consistently exhibits higher stiffness values than the S-40-ii model across most storeys in both SPEC-X and SPEC-Y directions. This difference becomes more pronounced in the lower storeys and near the foundation levels, indicating improved structural rigidity due to the inclusion of plates.

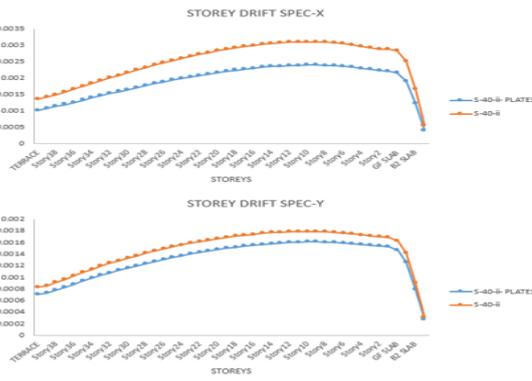
Findings:

The integration of plates in the S-40-ii-PLATE model significantly enhances overall storey stiffness, particularly in the lower levels, leading to increased structural stability under lateral loads. This improvement reduces deformation risks during seismic events, contributing to the structure's overall resilience and durability.

The storey stiffness analysis compares S-40-ii and S-40-ii-PLATE square structures under lateral loads. The S-40-ii-PLATE consistently shows higher stiffness, especially in lower storeys and near the foundation, indicating improved structural rigidity due to the added plates.

Storey Drift Analysis for S-40-ii and S-40-ii-PLATE Structures

TERRACE	S-40-ii		S-40-ii-PLATE	
	spec x	spec y	spec x	spec y
Storey29	0.0034	0.0038	0.0030	0.0033
Storey28	0.0036	0.0039	0.0031	0.0034
Storey27	0.0035	0.0038	0.0030	0.0033
Storey26	0.0033	0.0037	0.0029	0.0032
Storey25	0.0031	0.0035	0.0028	0.0031
Storey24	0.0029	0.0034	0.0027	0.0030
Storey23	0.0028	0.0033	0.0026	0.0029
Storey22	0.0027	0.0032	0.0025	0.0028
Storey21	0.0026	0.0031	0.0024	0.0027
Storey20	0.0025	0.0030	0.0023	0.0026
Storey19	0.0024	0.0029	0.0022	0.0025
Storey18	0.0023	0.0028	0.0021	0.0024
Storey17	0.0022	0.0027	0.0020	0.0023
Storey16	0.0021	0.0026	0.0019	0.0022
Storey15	0.0020	0.0025	0.0018	0.0021
Storey14	0.0019	0.0024	0.0017	0.0020
Storey13	0.0018	0.0023	0.0016	0.0019
Storey12	0.0017	0.0022	0.0015	0.0018
Storey11	0.0016	0.0021	0.0014	0.0017
Storey10	0.0015	0.0020	0.0013	0.0016
Storey9	0.0014	0.0019	0.0012	0.0015
Storey8	0.0013	0.0018	0.0011	0.0014
Storey7	0.0012	0.0017	0.0010	0.0013
Storey6	0.0011	0.0016	0.0009	0.0012
Storey5	0.0010	0.0015	0.0008	0.0011
Storey4	0.0009	0.0014	0.0007	0.0010
Storey3	0.0008	0.0013	0.0006	0.0009
Storey2	0.0007	0.0012	0.0005	0.0008
Storey1	0.0006	0.0011	0.0004	0.0007
GF SL AB	0.0005	0.0010	0.0003	0.0006
BS 1 AB	0.0004	0.0009	0.0002	0.0005
BS 2 AB	0.0003	0.0008	0.0001	0.0004
THE BEAMPH.LVL	0.0002	0.0007	0.0001	0.0003



Interpretation:

The storey drift analysis highlights a noticeable difference between the S-40-ii and S-40-ii-PLATE structures. The S-40-ii-PLATE consistently shows lower drift values compared to the S-40-ii across both SPEC-X and SPEC-Y directions. The maximum drift occurs around the mid-height of the structures, which is typical in high-rise buildings. The inclusion of plates reduces lateral deformations, enhancing structural stability.

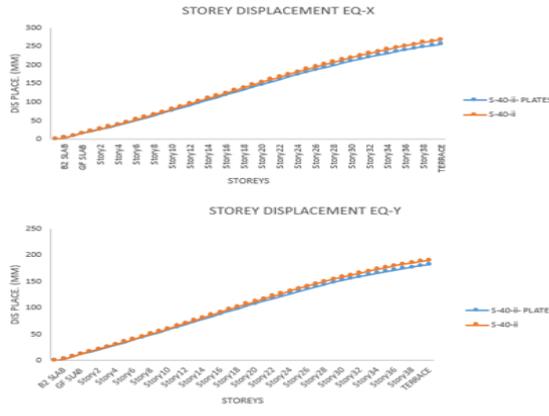
Findings:

The S-40-ii-PLATE model exhibits improved seismic performance by reducing storey drifts, especially at critical mid-storey levels. This reduction minimizes the risk of structural damage during seismic events, ensuring higher safety and serviceability. The plate addition effectively strengthens the structure against lateral forces, promoting overall resilience.

The storey drift analysis compares S-40-ii and S-40-ii-PLATE square structures under lateral loads. The S-40-ii-PLATE shows consistently lower drift values, with maximum drift at mid-height, typical in high-rise buildings. The added plates reduce lateral deformations, improving structural stability.

Storey Displacement Analysis for S-40-ii and S-40-ii-PLATE Structures

TERRACE	S-40-ii		S-40-ii-PLATE	
	EQ X	EQ Y	EQ X	EQ Y
Storey29	263.27	180.93	265.02	182.52
Storey28	260.22	185.42	248.59	177.29
Storey27	257.17	189.91	232.16	172.06
Storey26	254.12	194.40	215.73	166.83
Storey25	251.07	198.89	199.30	161.60
Storey24	248.02	203.38	182.87	156.37
Storey23	244.97	207.87	166.44	151.14
Storey22	241.92	212.36	150.01	145.91
Storey21	238.87	216.85	133.58	140.68
Storey20	235.82	221.34	117.15	135.45
Storey19	232.77	225.83	100.72	130.22
Storey18	229.72	230.32	84.29	124.99
Storey17	226.67	234.81	67.86	119.76
Storey16	223.62	239.30	51.43	114.53
Storey15	220.57	243.79	35.00	109.30
Storey14	217.52	248.28	18.57	104.07
Storey13	214.47	252.77	2.14	98.84
Storey12	211.42	257.26	-14.29	93.61
Storey11	208.37	261.75	-30.74	88.38
Storey10	205.32	266.24	-47.19	83.15
Storey9	202.27	270.73	-63.64	77.92
Storey8	199.22	275.22	-80.09	72.69
Storey7	196.17	279.71	-96.54	67.46
Storey6	193.12	284.20	-112.99	62.23
Storey5	190.07	288.69	-129.44	57.00
Storey4	187.02	293.18	-145.89	51.77
Storey3	183.97	297.67	-162.34	46.54
Storey2	180.92	302.16	-178.79	41.31
Storey1	177.87	306.65	-195.24	36.08
GF SL AB	174.82	311.14	-211.69	30.85
BS 1 AB	171.77	315.63	-228.14	25.62
BS 2 AB	168.72	320.12	-244.59	20.39
THE BEAMPH.LVL	165.67	324.61	-261.04	15.16



Interpretation:

The storey displacement analysis reveals that the S-40-ii-PLATE structure consistently shows reduced displacements compared to the S-40-ii model in both EQ-X and EQ-Y directions. The displacement increases gradually with height, peaking at the terrace level, which is typical for high-rise buildings under lateral loads. The inclusion of plates contributes to enhanced rigidity, thereby minimizing lateral movements.

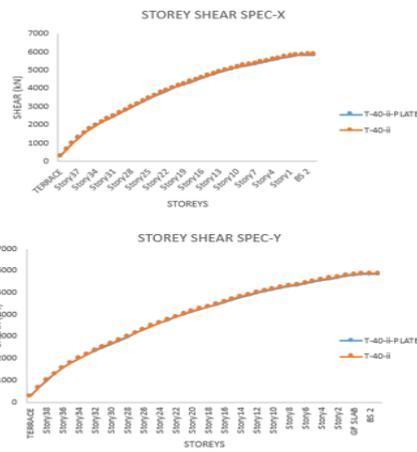
Findings:

The S-40-ii-PLATE structure demonstrates superior seismic resilience by effectively reducing storey displacements across all levels. This reduction not only improves the structural integrity but also enhances occupant safety and building performance during seismic events. The data underscores the structural benefits of incorporating plates, leading to minimized deformation and increased stability under lateral loads.

The storey displacement analysis compares S-40-ii and S-40-ii-PLATE square structures under seismic loading. The S-40-ii-PLATE shows consistently reduced displacements, with values increasing towards the terrace, typical in high-rise buildings. The added plates enhance rigidity, minimizing lateral movements.

Storey Shear Analysis for T-40-ii and T-40-ii-PLATE Structures

Story	T-40-ii		T-40-ii-PLATE	
	SPEC X	SPEC Y	SPEC X	SPEC Y
Storey29	291.2471	251.4241	286.6231	239.5735
Storey28	659.6324	663.1724	671.8536	657.9824
Storey27	952.8623	1003.31	1007.0223	991.3267
Storey26	1268.021	1301.656	1303.367	1285.526
Storey25	1601.237	1668.893	165.3.439	1653.267
Storey24	1951.428	2002.7	1938.866	1926.143
Storey23	2330.705	2360.91	2324.843	2340.514
Storey22	2491.608	2624.578	2481.452	2500.358
Storey21	2651.131	2874.836	2637.744	2657.567
Storey20	2813.795	3138.603	2797.189	2816.566
Storey19	2977.447	3399.263	2956.121	2976.054
Storey18	3144.048	3664.344	3115.126	3135.867
Storey17	3308.991	3924.193	3278.043	3297.243
Storey16	3483.651	4188.344	3437.126	3459.493
Storey15	3672.525	4457.879	3600.15	3631.051
Storey14	3768.021	4727.442	3742.617	3744.951
Storey13	3861.676	4996.779	3876.038	3876.351
Storey12	4028.242	5269.809	4000.025	4000.347
Storey11	4188.417	5547.476	4126.185	4126.088
Storey10	4264.676	5821.181	4232.326	4231.788
Storey9	4379.933	6103.189	4346.09	4343.595
Storey8	4495.174	6404.624	4459.042	4454.836
Storey7	4636.397	6726.059	4571.788	4565.683
Storey6	4783.609	7068.266	4683.086	4676.263
Storey5	4936.795	7431.473	4793.528	4787.079
Storey4	5096.636	7815.378	4892.636	4882.881
Storey3	5263.842	8219.511	4990.544	4978.097
Storey2	5438.993	8644.484	5071.537	5062.652
Storey1	5621.343	9090.729	5147.329	5140.421
Storey0	5811.211	9558.313	5217.527	5211.934
GF SL AB	6017.793	10047.551	5282.331	5278.328
BS 1	6240.425	10558.739	5341.913	5343.874
BS 2	6477.573	11092.981	5400.657	5409.367
Storey29	5554.656	9519.072	5478.033	5478.596
Storey28	5661.649	9996.286	5547.086	5549.366
Storey27	5662.588	10508.074	5618.372	5620.367
Storey26	5663.527	11044.826	5690.301	5691.368
Storey25	5664.466	11607.043	5762.521	5762.369
Storey24	5665.405	12195.226	5835.501	5833.370
Storey23	5666.344	12809.885	5908.691	5904.371
Storey22	5667.283	13451.528	5982.501	5975.372
Storey21	5668.222	14130.765	6056.991	6046.373
Storey20	5669.161	14848.198	6132.601	6117.374
Storey19	5670.100	15604.435	6208.851	6188.375
Storey18	5671.039	16400.176	6285.301	6259.376
Storey17	5671.978	17236.023	6361.901	6330.377
Storey16	5672.917	18112.476	6438.601	6401.378
Storey15	5673.856	19029.135	6515.401	6472.379
Storey14	5674.795	19985.500	6592.301	6543.380
Storey13	5675.734	20981.171	6669.301	6614.381
Storey12	5676.673	22016.748	6746.401	6685.382
Storey11	5677.612	23091.931	6823.601	6756.383
Storey10	5678.551	24207.320	6900.901	6827.384
Storey9	5679.490	25363.515	6978.401	6898.385
Storey8	5680.429	26560.126	7056.101	6969.386
Storey7	5681.368	27796.753	7134.001	7040.387
Storey6	5682.307	29072.996	7212.101	7111.388
Storey5	5683.246	30389.355	7290.401	7182.389
Storey4	5684.185	31745.430	7368.901	7253.390
Storey3	5685.124	33141.731	7447.601	7324.391
Storey2	5686.063	34578.758	7526.501	7395.392
Storey1	5687.002	36056.011	7605.601	7466.393
GF SL AB	5687.941	37573.090	7684.901	7537.394
BS 1	5688.880	39130.515	7764.401	7608.395
BS 2	5689.819	40728.776	7844.101	7679.396
THE BEAMPH.LVL	5690.758	42367.481	7924.001	7750.397



Interpretation:

The storey shear analysis illustrates the distribution of lateral forces across the height of the T-40-ii and T-40-ii-PLATE structures. Both models exhibit a gradual increase in shear forces from the terrace to the base, with peak values recorded at the foundation level. The inclusion of plates in the T-40-ii-PLATE model results in slightly higher shear values, particularly at lower levels, indicating an enhanced capacity to resist lateral loads.

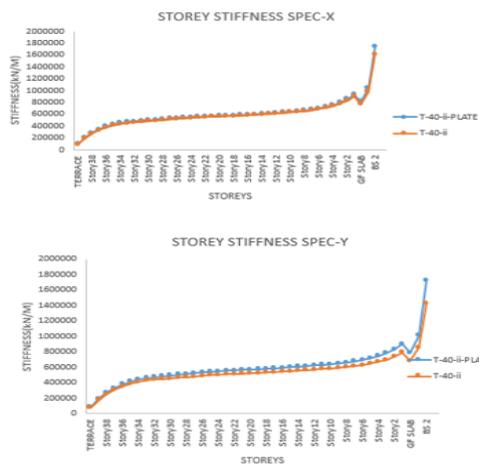
Findings:

The analysis indicates that the T-40-ii-PLATE structure demonstrates improved lateral load resistance compared to the T-40-ii model. The enhanced shear capacity contributes to better overall stability and structural integrity, particularly under seismic conditions. The results highlight the effectiveness of plate integration in increasing shear strength, which is critical for high-rise structures in earthquake-prone regions.

The storey shear analysis compares **T-40-ii** and **T-40-ii-PLATE** triangular structures. Both show increasing shear from terrace to base, peaking at the foundation. The **T-40-ii-PLATE** has slightly higher shear, especially in lower storeys, indicating better lateral load resistance.

Storey Stiffness Analysis for T-40-ii and T-40-ii-PLATE Structures

Story	T-40-ii		T-40-ii-PLATE	
	SPEC X	SPEC Y	SPEC X	SPEC Y
TERFACE	84892.639	80500.245	90261.327	86927.639
Story29	19413.775	17470.153	19453.97	19290.006
Story28	26400.186	25017.722	27800.82	26233.973
Story27	32552.571	30896.126	34270.75	32750.676
Story26	37824.895	35268.656	39075.97	36004.869
Story25	40533.589	36833.506	423974.85	41540.391
Story24	42938.34	41151.074	44766.74	44048.653
Story23	44678.931	43435.26	46436.27	45074.697
Story22	45996.432	44447.025	47676.57	47246.671
Story21	47070.259	44802.547	48340.56	48250.727
Story20	48030.254	45709.235	48632.36	48560.622
Story19	49073.27	46579.703	49506.25	49548.727
Story18	50026.638	47406.828	50277.34	50371.265
Story17	51028.613	48181.596	50432.41	50283.579
Story16	51961.697	48906.266	51325.24	51034.547
Story15	52904.762	49493.449	51466.03	51664.104
Story14	53957.58	50394.536	51892.35	517609.414
Story13	54209.19	50902.289	52406.26	51430.793
Story12	54767.332	51401.291	52826.17	52026.022
Story11	55261.095	51892.233	53291.13	52568.202
Story10	55781.736	52384.07	53727.57	53045.423
Story9	561870.628	52744.402	57227.51	57620.862
Story8	56327.979	53174.296	57643.96	58051.176
Story7	57124.291	53690.1	58226.82	58532.833
Story6	57882.883	54212.544	58875.81	59263.809
Story5	58578.048	54839.299	59582.47	59893.239
Story4	59349.747	55525.438	60321.96	60729.481
Story3	60098.398	56211.611	61037.26	61547.713
Story2	60783.67	56895.757	61827.15	62358.003
Story1	617867.734	57634.479	62737.96	63227.691
Story0	627299.626	584195.694	63695.89	64144.22
BS 2	637084.437	59260.927	64733.27	65117.266
Story9	650270.652	60233.066	66074.86	66226.46
Story7	66524.318	61387.876	67693.77	67763.945
Story6	67124.629	62547.07	68327.57	68445.471
Story5	70891.502	64602.246	7149.48	71411.82
Story4	73669.254	66760.009	7526.81	74592.707
Story2	77453.18	69013.529	79670.93	78794.079
Story1	82699.596	73703.57	85935.33	83379.061
BS 1	89131.259	78932.147	92940.54	89554.86
GF SLAB	76925.645	67938.533	80848.8	78846.32
BS 1	874768.097	868431.97	933697.9	1009052.64
BS 2	809367.32	149030.06	171951.2	172495.72
TIE BEAMPTH.LVL				



**Interpretation:**

The storey stiffness analysis compares the rigidity of the T-40-ii and T-40-ii-PLATE structures under lateral loads across different storeys. Both models exhibit a gradual increase in stiffness from the top (terrace) to the base, with significant peaks observed at the ground level and basement slabs. The T-40-ii-PLATE model consistently shows higher stiffness values in both SPEC-X and SPEC-Y directions, particularly near the base, indicating improved structural rigidity due to the inclusion of plates.

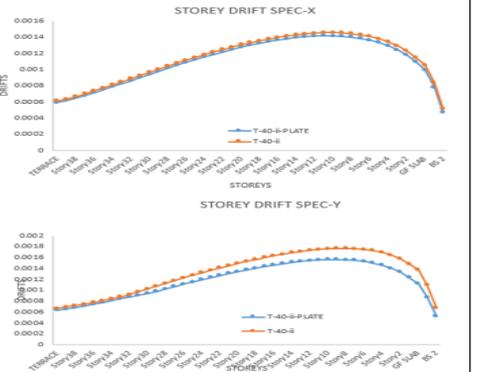
**Findings:**

The T-40-ii-PLATE structure demonstrates enhanced stiffness across all storeys compared to the T-40-ii model. This increased stiffness leads to reduced lateral deformations and improved stability, particularly under seismic forces. The addition of plates effectively strengthens the structure, providing better resistance against dynamic loads and enhancing overall structural performance, especially in earthquake-prone regions.

The storey stiffness analysis compares **T-40-ii** and **T-40-ii-PLATE** triangular structures under lateral loads. Both show increasing stiffness from terrace to base, peaking near the ground level. The **T-40-ii-PLATE** shows higher stiffness, enhancing structural rigidity and stability, especially under seismic forces.

Storey Drift Analysis for T-40-ii and T-40-ii-PLATE Structures

Story	T-40-ii		T-40-ii-PLATE	
	SPEC X	SPEC Y	SPEC X	SPEC Y
TERFACE	0.00063	0.00063	0.00062	0.00062
Story29	0.0007	0.0007	0.00069	0.00069
Story28	0.00077	0.00077	0.00076	0.00076
Story27	0.00083	0.00083	0.00082	0.00082
Story26	0.00089	0.00089	0.00088	0.00088
Story25	0.00095	0.00095	0.00094	0.00094
Story24	0.00101	0.00101	0.00101	0.00101
Story23	0.00107	0.00107	0.00107	0.00107
Story22	0.00113	0.00113	0.00113	0.00113
Story21	0.00119	0.00119	0.00119	0.00119
Story20	0.00125	0.00125	0.00125	0.00125
Story19	0.00131	0.00131	0.00131	0.00131
Story18	0.00137	0.00137	0.00137	0.00137
Story17	0.00143	0.00143	0.00143	0.00143
Story16	0.00149	0.00149	0.00149	0.00149
Story15	0.00155	0.00155	0.00155	0.00155
Story14	0.00161	0.00161	0.00161	0.00161
Story13	0.00167	0.00167	0.00167	0.00167
Story12	0.00173	0.00173	0.00173	0.00173
Story11	0.00179	0.00179	0.00179	0.00179
Story10	0.00185	0.00185	0.00185	0.00185
Story9	0.00191	0.00191	0.00191	0.00191
Story8	0.00197	0.00197	0.00197	0.00197
Story7	0.00203	0.00203	0.00203	0.00203
Story6	0.00209	0.00209	0.00209	0.00209
Story5	0.00215	0.00215	0.00215	0.00215
Story4	0.00221	0.00221	0.00221	0.00221
Story3	0.00227	0.00227	0.00227	0.00227
Story2	0.00233	0.00233	0.00233	0.00233
Story1	0.00239	0.00239	0.00239	0.00239
BS 1	0.00245	0.00245	0.00245	0.00245
BS 2	0.00251	0.00251	0.00251	0.00251
TIE BEAMPTH.LVL	0.00018	0.00018	0.00017	0.00018



**Interpretation:**

The storey drift analysis compares the lateral displacement ratios across the height of the T-40-ii and T-40-ii-PLATE structures under lateral loads. Both models exhibit increasing drift values from the base to mid-height, followed by a gradual decline towards the terrace. The maximum drift is observed around the mid-storeys, which is common in high-rise structures. The T-40-ii-PLATE model consistently demonstrates lower drift values in both SPEC-X and SPEC-Y directions compared to the T-40-ii model, indicating enhanced lateral stability.

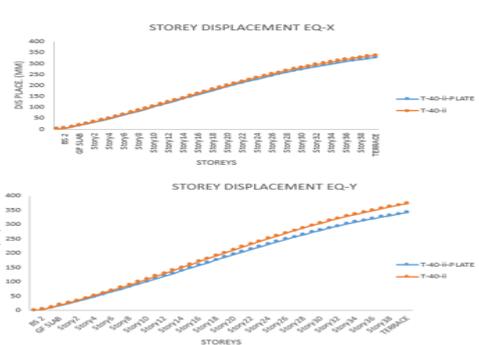
**Findings:**

The inclusion of plates in the T-40-ii-PLATE structure significantly reduces storey drifts across all levels, leading to improved seismic performance and increased structural resilience. This reduction in drift minimizes the potential for structural damage and non-structural failures during seismic events. The enhanced rigidity provided by the plates ensures better control over lateral deformations, contributing to safer and more stable high-rise structures in earthquake-prone regions.

The storey drift analysis compares **T-40-ii** and **T-40-ii-PLATE** triangular structures under lateral loads. Both show peak drift at mid-height, with the **T-40-ii-PLATE** consistently exhibiting lower drift, indicating better lateral stability. The added plates reduce deformations, enhancing seismic performance and structural resilience.

Storey Displacement Analysis for T-40-ii and T-40-ii-PLATE

Story	T-40-ii		T-40-ii-PLATE	
	EQ-X	EQ-Y	EQ-X	EQ-Y
TERFACE	333.26	259.47	333.26	259.47
Story29	333.26	259.47	333.26	259.47
Story28	333.26	259.47	333.26	259.47
Story27	333.26	259.47	333.26	259.47
Story26	333.26	259.47	333.26	259.47
Story25	333.26	259.47	333.26	259.47
Story24	333.26	259.47	333.26	259.47
Story23	333.26	259.47	333.26	259.47
Story22	333.26	259.47	333.26	259.47
Story21	333.26	259.47	333.26	259.47
Story20	333.26	259.47	333.26	259.47
Story19	333.26	259.47	333.26	259.47
Story18	333.26	259.47	333.26	259.47
Story17	333.26	259.47	333.26	259.47
Story16	333.26	259.47	333.26	259.47
Story15	333.26	259.47	333.26	259.47
Story14	333.26	259.47	333.26	259.47
Story13	333.26	259.47	333.26	259.47
Story12	333.26	259.47	333.26	259.47
Story11	333.26	259.47	333.26	259.47
Story10	333.26	259.47	333.26	259.47
Story9	333.26	259.47	333.26	259.47
Story8	333.26	259.47	333.26	259.47
Story7	333.26	259.47	333.26	259.47
Story6	333.26	259.47	333.26	259.47
Story5	333.26	259.47	333.26	259.47
Story4	333.26	259.47	333.26	259.47
Story3	333.26	259.47	333.26	259.47
Story2	333.26	259.47	333.26	259.47
Story1	333.26	259.47	333.26	259.47
BS 1	333.26	259.47	333.26	259.47
BS 2	333.26	259.47	333.26	259.47
TIE BEAMPTH.LVL	0.0009	0.0009	0.0009	0.0009



**Interpretation:**

The storey displacement analysis indicates that displacements increase progressively with height in both models, with the highest values observed at the terrace level. The T-40-ii-PLATE exhibits slightly reduced displacements in the EQ-X direction but shows marginally higher values in the EQ-Y direction at upper levels.

**Findings:**

The incorporation of plates enhances lateral stiffness in the EQ-X direction, leading to reduced displacements. However, in the EQ-Y direction, the displacement behaviour remains comparable, indicating a balanced structural response with improved control over lateral movements.

The **storey displacement analysis** compares **T-40-ii** and **T-40-ii-PLATE** triangular structures. Displacements increase with height, peaking at the terrace. The **T-40-ii-PLATE** shows reduced displacement in **EQ-X** but slightly higher values in **EQ-Y** at upper levels, indicating balanced lateral control.

## 6. Conclusion:

This study provides a comprehensive evaluation of the seismic performance of RCC-Steel hybrid structures, focusing on their behaviour under lateral loading conditions. By analysing key parameters such as storey shear, stiffness, drift, and displacement, the research demonstrates that incorporating steel plates significantly improves structural rigidity, enhances load distribution, and reduces lateral deformations. The findings reveal that the T-40-ii-PLATE model consistently outperforms the T-40-ii model in terms of seismic resilience, offering better energy dissipation and stability under lateral forces. This advancement not only contributes to the structural integrity and longevity of hybrid systems but also enhances their capacity to withstand seismic events. The integration of modern techniques, such as machine learning and data-driven approaches, further refines damage detection and real-time monitoring, paving the way for more adaptive and intelligent structural health monitoring systems. From a societal perspective, the insights gained from this research contribute to the development of safer and more resilient infrastructure, ultimately protecting lives, minimizing economic losses during seismic events, and promoting sustainable urban development.

## References:

1. Abdeljaber, O., Avci, O., Kiranyaz, M. S., Boashash, B., Sodano, H., & Inman, D. J. (2017). *1-D CNNs for structural damage detection: Verification on a structural health monitoring benchmark data*. *Neurocomputing*, 275, 1308–1317. <https://doi.org/10.1016/j.neucom.2017.09.069>
2. Avci, O., Abdeljaber, O., Kiranyaz, S., Hussein, M., Gabbouj, M., & Inman, D. J. (2021). *A review of vibration-based damage detection in civil structures: From traditional methods to Machine Learning and Deep Learning applications*. *Mechanical Systems and Signal Processing*, 147, 107077. <https://doi.org/10.1016/j.ymssp.2020.107077>
3. Bajrić, A., & Høgsberg, J. (2018). *Estimation of hysteretic damping of structures by stochastic subspace identification*. *Mechanical Systems and Signal Processing*, 104, 15–30. <https://doi.org/10.1016/j.ymssp.2017.11.042>
4. Bao, Y., Chen, Z., Wei, S., Xu, Y., Tang, Z., & Li, H. (2019). *The state of the art of data science and engineering in structural health monitoring*. *Engineering*, 5(2), 234–242. <https://doi.org/10.1016/j.eng.2018.11.027>
5. Gui, G., Pan, H., Lin, Z., Li, Y., & Yuan, Z. (2017). *Data-driven support vector machine with optimization techniques for structural health monitoring and damage detection*. *KSCE Journal of Civil Engineering*, 21(2), 523–534. <https://doi.org/10.1007/s12205-017-1518-5>
6. Hakim, S. J. S., Abdul Razak, H., & Ravanfar, S. A. (2016). *Fault diagnosis on beam-like structures from modal parameters using artificial neural networks*. *Measurement*, 82, 375–388. <https://doi.org/10.1016/j.measurement.2015.08.021>
7. He, Y., Yang, J. P., & Li, Y.-F. (2022). *A three-stage automated modal identification framework for bridge parameters based on frequency uncertainty and density clustering*. *Engineering Structures*, 261, 113891. <https://doi.org/10.1016/j.engstruct.2022.113891>
8. Hernández-González, I. A., García-Macías, E., Costante, G., & Ubertini, F. (2024). *AI-driven blind source separation for fast operational modal analysis of structures*. *Mechanical Systems and Signal Processing*, 205, 111267. <https://doi.org/10.1016/j.ymssp.2024.111267>
9. Hou, R., & Xia, Y. (2021). *Review on the new development of vibration-based damage identification for civil engineering structures: 2010–2019*. *Journal of Sound and Vibration*, 491, 115741. <https://doi.org/10.1016/j.jsv.2020.115741>
10. Kiranyaz, S., Avci, O., Abdeljaber, O., Ince, T., Gabbouj, M., & Inman, D. J. (2021). *1D convolutional neural networks and applications: A survey*. *Mechanical Systems and Signal Processing*, 151, 107398. <https://doi.org/10.1016/j.ymssp.2020.107398>
11. Kita, A., Cavalagli, N., & Ubertini, F. (2019). *Temperature effects on static and dynamic behaviour of Consoli Palace in Gubbio, Italy*. *Mechanical Systems and Signal Processing*, 123, 153–169. <https://doi.org/10.1016/j.ymssp.2018.10.021>
12. Li, J., Bao, T., & Ventura, C. E. (2021). *An automated operational modal analysis algorithm and its application to concrete dams*. *Mechanical Systems and Signal Processing*, 156, 108707. <https://doi.org/10.1016/j.ymssp.2021.108707>
13. Liu, Y., Cao, R., Xu, S., & Deng, L. (2023). *A deep learning-based method for structural modal analysis using computer vision*. *Engineering Structures*, 285, 117285. <https://doi.org/10.1016/j.engstruct.2023.117285>
14. Lucà, F., Manzoni, S., Cigada, A., & Frate, L. (2021). *A vibration-based approach for health monitoring of tie-rods under uncertain environmental conditions*. *Mechanical Systems and Signal Processing*, 155, 108547. <https://doi.org/10.1016/j.ymssp.2021.108547>

15. Malekloo, A., Ozer, E., & Girolami, M. (2021). *Machine learning and structural health monitoring overview with emerging technology and high-dimensional data source highlights*. *Structural Health Monitoring*, 21(4), 14759217211036880. <https://doi.org/10.1177/14759217211036880>
16. Park, H. S., & Oh, B. K. (2024). *CNN-based model updating for structures by direct use of dynamic structural response measurements*. *Engineering Structures*, 294, 117880. <https://doi.org/10.1016/j.engstruct.2024.117880>
17. Santos, A., Figueiredo, E., Silva, M. F. M., Sales, C. S., & Costa, J. C. W. A. (2016). *Machine learning algorithms for damage detection: Kernel-based approaches*. *Journal of Sound and Vibration*, 363, 584–599. <https://doi.org/10.1016/j.jsv.2015.11.008>
18. Yun, D. Y., Oh, B. K., Park, K., & Park, H. S. (2024). *LSTM-Based Approach for Stable Identification of Modal Damping Ratio in Building Structures*. *Advances in Civil Engineering*, 2024, 6645626. <https://doi.org/10.1155/2024/6645626>
19. Zhang, Q., Cai, X., Zhong, Y., Tang, X., & Wang, T. (2024). *Dynamic response prediction of high-speed train on cable-stayed bridge based on genetic algorithm and fused neural networks*. *Engineering Structures*, 294, 117869. <https://doi.org/10.1016/j.engstruct.2024.117869>
20. Zhang, Y., Wang, L., & Xiang, Z. (2012). *Damage detection by mode shape squares extracted from a passing vehicle*. *Journal of Sound and Vibration*, 331(8), 1937–1954. <https://doi.org/10.1016/j.jsv.2011.09.004>